

# Experimental Study of Wave Attenuation for a Tandem Breakwater

Nur Aini Mohd Arish, Othman A. Karim, Wan Hanna Melini Wan Mohtar

**Abstract**— This paper experimentally investigates the wave attenuation of a tandem breakwater. Tandem breakwater is conventional rubble mound breakwater sheltered with a seaward-submerged structure. The experimental works were done in a wave basin with dimensions of 25 m length, 18 m width and 1.2 m height. All tests were performed with regular waves generated from a piston type multi element wave maker with wave height,  $H = 0.15, 0.18$  and  $0.20$  m and wave period,  $T = 2.05, 2.20$  and  $2.50$  sec. Wave gauge was positioned at eight different locations to record water level and by using the measured data, the wave attenuation, WHA was calculated ( $WHA = 1 - K_t$ ). The objective of the experiment was to study the wave attenuation for the tandem breakwater with the influence of various angle of wave attack,  $0, 15, 30$  and  $60$  degree. Apart from the influence of angle of wave attack, the influence of relative distance between submerged and rubble mound breakwater towards wave attenuation, WHA was also been investigated. Tests are done for various breakwater spacing, ( $X/d = 8.33-15.56$ ) and relative heights ( $h/d = 0.42-0.56$ ). The results achieved shows that WHA are increasing along with the increasing of angle of wave attack but it is vice versa with increasing of the water depth. The highest WHA is 48.46 percent and 55.02 percent for relative distance,  $X/d = 6.67-8.89$  (4 m) and  $X/d = 10.0-13.33$  (6 m) respectively, both at the condition of  $60^\circ$  angle of wave attack.

**Index Terms**— tandem breakwater, wave attack, wave height attenuation, wave transmission.

## I. INTRODUCTION

Over years, breakwater has known as a coastal defensive structure, which are built to protect ports and harbours from wave attack, to maintain a calm condition and as a safeguard to beaches. Submerged breakwaters are becoming a popular option for coastal protection is because of their low aesthetic effect on the natural environment [1]. Tandem breakwater is a combination of main conventional rubble mound breakwater and submerged structure in front of it. The unique of this combination of breakwater is that most of the wave energy is dissipated and wave intensity is reduced by the submerged structure. The submerged structure effectively trips the steeper waves and dissipates a major portion of the wave energy. Smaller waves will develop and interact with the

main breakwater and ensure its stability [2], [3]. One of the main aspect in measuring the performance of submerged breakwater is the transmission coefficient. The wave transmission coefficient,  $K_t$  is defined as the ratio of the transmitted wave height ( $H_t$ ) at the leeside to the incident wave height ( $H_i$ ) at the breakwater seaward ([4]:

$$K_t = \frac{H_t}{H_i}$$

Common parameters that influence the value of the wave transmission coefficient are the crest width of the breakwater ( $B$ ), the height of the structure ( $h_c$ ), the slope of the submerged structure ( $m$ ), the nominal rock diameter of the armour slope ( $D_{n50}$ ), the freeboard ( $R_c$ ), the water depth ( $h$ ), the incident significant wave height ( $H_i$ ), the wave period ( $T$ ) and the wave steepness ( $s$ ) [5]–[8]. Figure 1 gives a graphical representation of these parameters.

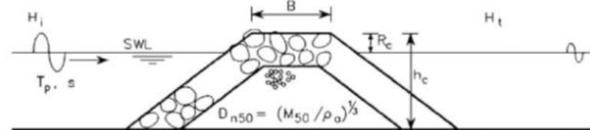


Figure 1: Parameters effecting the value of the transmission coefficient [9].

When the value of  $K_t$  is approaching 0, it indicates that there is no transmission occurred due to the impermeable or high breakwaters. If there is no reduction in wave height, the  $K_t$  will be 1 ([10]. Wave height attenuation (WHA) can be expressed as  $(1 - K_t) \times 100$  [2].

## II. LITERATURE REVIEW

One of the earliest applications of tandem breakwater is by Cox and Clark [11]. A breakwater defended by seaward-submerged reef structure has been constructed for protecting a marina harbour. They applied the relative breakwater spacing ( $X/d$ ) ranging from 3.49 to 5.81. A series of tests were done and maximum wave height attenuation of 32% was resulted. Another research was conducted by applying tandem breakwater concept [2], [12]. The study used breakwater spacing ( $X/d$ ) ranging from 3.33 to 4.29 and resulted a maximum of 25% wave height attenuation. Shirlal et al. [13] conducted study for ( $X/d = 2.5-13.33$ ) and for different relative heights ( $h/d = 0.625-0.833$ ) and relative widths ( $B/d = 0.25-1.33$ ) of the submerged structure. It was observed that with ( $B/d = 0.6-0.75$ ) constructed and relative distance ( $X/d$ ) of 6.25–8.33 breaks all the incoming waves and dissipates energy and protects the breakwater optimally.

Most of the studies involving wave transmission are using 2D flume and this only focus on wave that is perpendicular to the structure in the flume [8]. It was found that the values of

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wave transmission  $K_t$  are decreasing when there is an increase of incident angle of wave attack [14], [15]. For this paper, a study was done to measure the wave height attenuation for a tandem breakwater system with the influence of various wave attack that is not perpendicular to the structure.

III. MATERIALS AND METHODS

The experiments are carried out in a wave basin at the National Hydraulic Research Institute of Malaysia (NAHRIM). The wave basin consists of a conventional rubble mound breakwater and submerged breakwater as shown in Figure 2. The wave basin is 25 m long, 18 m wide and 1.2 m deep. Figure 3 shows the location of the wave gauges for the measurement of the incident and transmitted wave, a wave generator at one end and a wave absorber at the other. The conventional breakwater is constructed on the flat bed of the wave basin with a model scale 1:20. The submerged breakwater is built with crest width 0.3 and at location 4 m and 6 m seaward of the conventional breakwater. The tandem breakwater model is tested for wave heights of 0.15m, 0.18 m and 0.20 m in water depths of 0.45, 0.50, and 0.60 m. Regular waves with various heights, water depth and of periods 2.05s to 2.50s can be generated with this facility. Table 1-4 details out the characteristic values for the conventional and submerged breakwater used in the tandem breakwater system.

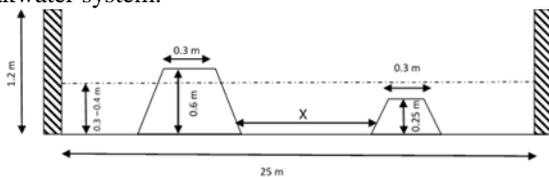


Figure 2: Side view plan of the tandem breakwater system

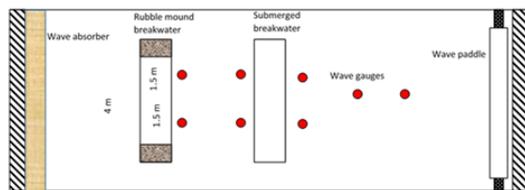


Figure 3: The arrangement of the wave gauge in the wave basin

Table 1: Wave characteristics

Characteristics	Value
Wave height, $H$	0.15m, 0.18 m and 0.20 m
Wave period, $T$	2.05s, 2.20s and 2.50s
Number of waves, $N$	1000 waves
Angle of wave attack, $\theta$	0°, 15°, 30° dan 60°
Water depth, $d$	0.45 m, 0.50 m and 0.60 m

Table 2: Characteristics for conventional breakwater model

Characteristics	Value
Model scale	1 : 20
Height	$H = 0.70$ m
Crest width	$B = 0.30$ m
Length	$L = 4$ m
Slope ratio	1 : 2
Armour layer	Quarry stone and mortar cube
Nominal diameter, $D_{n50}/D_n$	0.050 m – Stone 0.049 m – Cube
Weight, $W_{50}$	270-300 gm – Stone 282 gm – Cube

Table 3: Characteristics for submerged breakwater model

Characteristics	Value
Model scale	1 : 20
Height	$h = 0.25$ m
Crest width	$B = 0.30$ m
Length	$L = 4$ m
Side slope	1 : 2
Material	Quarry stones
Nominal diameter, $D_{n50}$	0.03 m
Weight, $W_{50}$	150-200 gm
Porosity	0.45
Distance between conventional breakwater and submerged breakwater, $X$	4.0 m and 6.0 m

Table 4: Dimensionless parameter for wave and submerged breakwater

Characteristics	Value
Relative height, $h/d$	0.42-0.56
Relative width, $B/d$	0.5-0.67
Relative submergence, $F/H_i$	1.00-2.33
Relative distance, $X/d$	8.33-15.56
Wave steepness, $H/gT^2$	0.0024-0.0044
Relative depth, $d/gT^2$	0.0073-0.015

IV. RESULTS AND DISCUSSION

A. Influence of relative depth,  $d/gT^2$  on wave height attenuation, WHA

Figure 4 indicated that as relative depth,  $d/gT^2$  increases, the wave height attenuation is decreasing. This is because when water depth increases, the submergence of structure increases and this reduces wave breaking resulting in increased

transmission. The figure shows the variation of wave height attenuation, *WHA* with the deep-water wave steepness parameter,  $H/gT^2$  for different relative depth,  $d/gT^2$  for the condition of  $0^\circ$  wave angle. The maximum wave height attenuation, *WHA* attained are 24.5% for 0.00734-0.01092(0.45m), 20.7% for 0.00816-0.01213(0.50m) and 10.4% for 0.00979-0.01455(0.60 m), respectively.

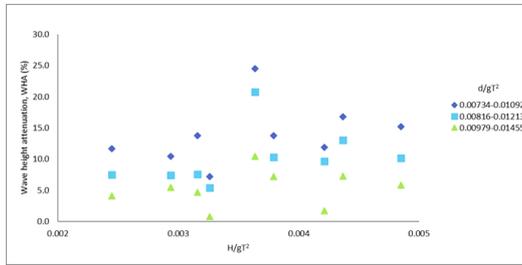


Figure 4: Wave height attenuation, *WHA* for different relative depth,  $d/gT^2$

**B. Influence of relative distance,  $X/d$  on wave height attenuation, *WHA*.**

The comparison value of wave height attenuation, *WHA* between relative distance,  $X/d= 6.67-8.89(4\text{ m})$  and  $10.0-13.33(6\text{ m})$  are shown in Figure 5. It is observed that *WHA* for the  $X/d = 10.0-13.33(6\text{ m})$  are higher than  $X/d 6.67-8.89(4\text{ m})$ . As the distance between breakwaters ( $X$ ) increases, the waves that break over the submerged breakwater, lose some more energy while propagating in the energy dissipation zone and causing higher wave energy dissipation. Results show that for  $X/d = 6.67-8.89(4\text{ m})$  with relative depth,  $d/gT^2 = 0.00734-0.01092(0.45\text{m})$  the wave height attenuation (*WHA*) is 24.5 percent (maximum) while for  $10.0-13.33(6\text{ m})$  the *WHA* is 42.0 percent (maximum) with the same relative depth. Research by [13] found out that with  $X/d = 2.5-3.33$ , it attenuates the waves by a maximum amount of 18%,  $X/d = 6.25-8.33$  at about 33% and for  $X/d = 10-13.33$  at about 43%.

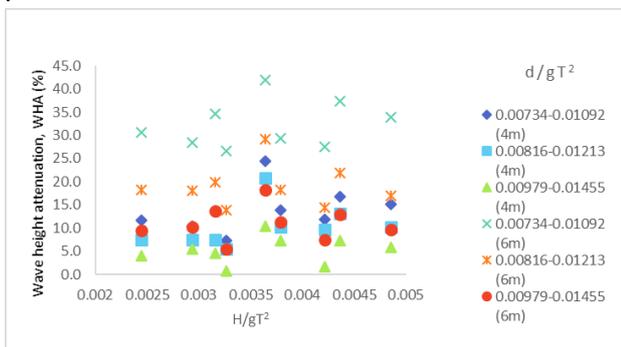


Figure 5: Wave height attenuation, *WHA* for relative distance,  $X/d 6.67-8.89(4\text{ m})$  and  $10.0-13.33(6\text{ m})$

**C. Influence of angle of wave attack,  $\theta$  on wave height attenuation, *WHA*.**

Figures 6 and 7 shows the value of *WHA* for relative distance,  $X/d = 6.67-8.89(4\text{ m})$  with the influence of angle of wave attack,  $\theta$ . For the maximum relative depth,  $d/gT^2 =$

$0.008904-0.013163(0.60\text{ m})$  as in Figure 6, the wave height attenuation is increasing with the angle of wave attack,  $\theta$ . The same trend of graph can be seen for the minimum relative depth,  $d/gT^2 0.006678-0.009872(0.45\text{ m})$ . The highest *WHA* is 28.15 percent and 48.46 percent for depth of 0.60 m and 0.45 m respectively, both at the condition of  $60^\circ$  angle of wave attack. Table 5 details out the *WHA* for every water depth for  $X/d = 6.67-8.89(4\text{ m})$ .

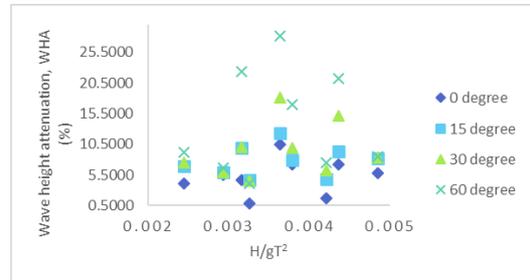


Figure 6: Location of submerged breakwater,  $X/d = 6.67-8.89(4\text{ m})$ , depth = 0.6m

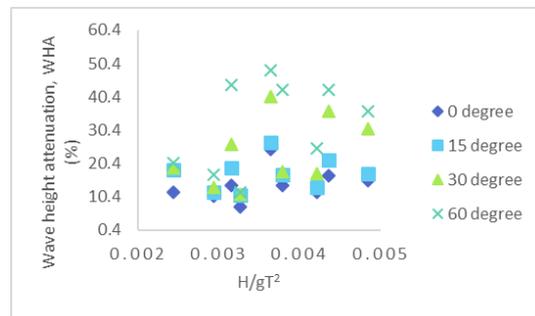


Figure 7: Location of submerged breakwater,  $X/d = 6.67-8.89(4\text{ m})$ , depth = 0.45m

Table 5: Wave height attenuation for relative distance,  $X/d = 6.67-8.89(4\text{ m})$

$d/gT^2$	Wave height attenuation ( <i>WHA</i> ) (%)			
	$0^\circ$	$15^\circ$	$30^\circ$	$60^\circ$
<b>0.00734-0.01092 (0.45m)</b>	24.	26.6	40.5	48.
<b>0.00816-0.01213 (0.50m)</b>	49	3	1	46
<b>0.00979-0.01455 (0.60m)</b>	20.	23.7	32.0	45.
	73	5	5	73
	10.	12.2	18.1	28.
	43	7	2	15

The same pattern of graph also can be seen for  $X/d = 10.0-13.33(6\text{ m})$ . All the graphs indicate the increasing value of *WHA* when the angle of wave attack,  $\theta$  are increasing from  $0^\circ$  to  $60^\circ$ . Figures 8 and 9 represents the graphs for the maximum relative depth,  $d/gT^2 = 0.008904-0.013163(0.60\text{ m})$  and minimum relative depth,  $d/gT^2 0.006678-0.009872(0.45\text{ m})$  respectively.

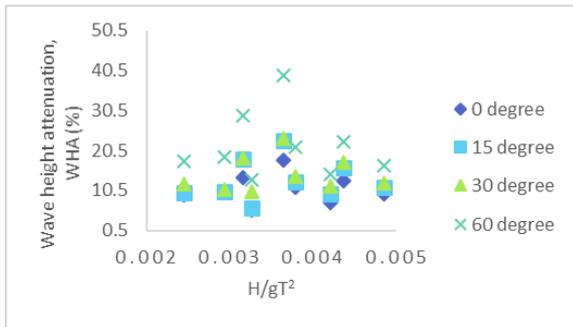


Figure 8: Location of submerged breakwater,  $X/d = 10.0-13.3$  (6 m), depth = 0.6m

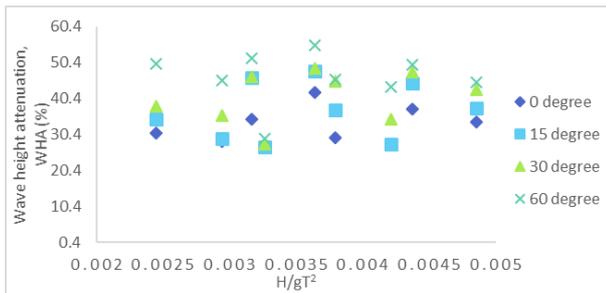


Figure 9: Location of submerged breakwater,  $X/d = 10.0-13.3$  (6 m), depth = 0.45m

Table 6 shows the detail results of *WHA* for relative distance,  $X/d = 10.0-13.3$  (6 m). It shows that *WHA* are increasing along with the increasing of angle of wave attack but it is decreasing with increasing of water depth. The highest *WHA* is for the depth of 0.45m and at the angle of 60°, which is 55.02%. The lowest *WHA* can be seen at the angle of 0° where the value is 18.19 percent.

Table 6: Wave height attenuation for relative distance,  $X/d = 10.0-13.33$  (6 m)

$d/gT^2$	Wave height attenuation ( <i>WHA</i> ) (%)			
	0°	15°	30°	60°
<b>0.00734-0.01092</b> <b>(0.45m)</b>	42.00	47.89	48.60	55.02
<b>0.00816-0.01213</b> <b>(0.50m)</b>	29.24	29.98	34.82	45.16
<b>0.00979-0.01455</b> <b>(0.60m)</b>	18.19	22.91	23.43	39.21

V. CONCLUSION

Following are the conclusions drawn from the present study:

1. As relative depth,  $d/gT^2$  increases, the wave height attenuation is decreasing. The highest *WHA* is 28.15 percent and 48.46 percent for depth of 0.60 m and 0.45 m respectively, for the condition of 60° angle of wave attack.
2. It is observed that *WHA* for the  $X/d = 10.0-13.33$ (6 m) are higher than  $X/d = 6.67-8.89$ (4 m). For  $X/d = 6.67-8.89$ (4 m) with relative depth,  $d/gT^2 = 0.00734-0.01092$ (0.45m) the wave height attenuation (*WHA*) is 24.5% (maximum) while for 10.0-13.33(6 m) the *WHA* is 42.0% (maximum) with the same relative depth.

3. *WHA* are increasing along with the increasing of angle of wave attack. For relative distance,  $X/d = 6.67-8.89$  (4 m) the *WHA* for 0.45 m depth is 24.49 for 0° wave attack and 48.46% for 60° wave attack

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